

EXTREME PRECIPITATION EVENTS AND ASSOCIATED RISK OF FAILURE IN HYDRAULIC PROJECTS IN THE STATE OF MATO GROSSO DO SUL, BRAZIL

EVENTOS EXTREMOS DE PRECIPITAÇÃO E O RISCO DE FALHA ASSOCIADO EM PROJETOS HIDRÁULICOS NO ESTADO DE MATO GROSSO DO SUL, BRASIL

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ABSTRACT

It is necessary to know the extreme precipitation occurrences in a region for satisfactory design of infrastructure projects. Tropical climate regions are characterized by heavy rainfall events during the summer, and, in recent years, as a result of climate change, such events are becoming recurrent. A theoretical probability distribution model is typically used to extrapolate extreme events for high return periods. This study verified if whether probability distribution models are efficient for estimation of extreme precipitation events for the normative recommended return period. Five cities in Mato Grosso do Sul State, in the Midwest region of Brazil, affected by disasters caused by heavy rainfall were adopted as a case study. The results illustrate that the recommendations of the return periods used for the design of hydraulic control structures are insufficient to avoid damage caused by precipitation events, thus lending a high breakdown risk to structures.

KEY WORDS: Maximum rainfall; Return period; Probabilistic analysis; Risk analysis; Lifetime structure; Probabilistic distribution model.

RESUMO

É necessário conhecer as ocorrências extremas de precipitação em uma região para obtenção de projetos de infraestrutura satisfatórios. As regiões de clima tropical são caracterizadas por seus eventos de chuvas intensas durante o verão e, nos últimos anos, em decorrência das mudanças climáticas, esses eventos estão se tornando recorrentes. Um modelo de distribuição de probabilidade teórica é normalmente aplicado para extrapolar eventos extremos para altos períodos de retorno. Neste estudo, verificou-se se os modelos de distribuição de probabilidade são eficientes para estimar eventos extremos de precipitação para o período de retorno recomendado em normativas de dimensionamento de obras hidráulicas. Cinco cidades do estado de Mato Grosso do Sul, localizadas na região centro-oeste do Brasil, que foram afetadas por desastres causados por fortes chuvas foram adotadas como estudo de caso. Os resultados ilustram que as recomendações dos períodos de retorno utilizados para o dimensionamento das estruturas de controle hidráulico são insuficientes para evitar os danos causados por eventos de precipitação, gerando um alto risco de ruptura das estruturas.

PALAVRAS CHAVE: Precipitação máxima; Período de retorno; Análise probabilística; Análise de risco; Vida útil da estrutura; Modelo de distribuição probabilística.



1. INTRODUCTION

The Intergovernmental Panel on Climate Change (IPCC, 2012) indicated that extreme precipitation events have been increasing over many locations on the planet. According to Salas et al. (2013), some of the changes observed in the hydrological records may be due to the effects of natural climatic variability, particularly resulting from low-frequency components of climate variability such as the El Niño Southern Oscillation (ENSO). Kyoung et al. (2011) predicted that rainfall patterns will change significantly as a result of climate change. Hajani et al. (2017) affirmed that a possible increase in rainfall intensity driven by climate change might increase the risk of floods in many locations worldwide. Extreme events, when not predicted in the design of hydraulic control structures, can cause significant social and economic damage.

Many researchers (Rahmani et al., 2014; Yang et al., 2010; Koutsoyiannis, 2004) have conducted rainfall frequency distribution studies. According to Babar and Ramesh (2014), the stochastic behaviour of extreme events can be analyzed based on their probability distribution function. Thus, probability models such as the Gumbel, log-normal (LN), log Pearson type III (LP3), and normal models can be used for fitting the frequency distribution of precipitation and flood data. In these cases, the intensity of the precipitation is associated with the probability of it being equaled or exceeded. The inverse of this probability defines a range of rainfall occurrences of the same or greater magnitude, called the return period (T), which is an average estimate of the time when an event is equaled or exceeded.

Muhammad et al. (2016) affirmed that return periods are useful in the field of hydrology to measure the severity of an event. In flood engineering practice, the return period has been defined as the average number of years to the first occurrence of a flood event of magnitude greater than a predefined design flood (Kite, 1977). The safety and durability of engineering projects are often associated with the return period adopted in the project. In the context of designing a hydraulic structure, return periods T are specified according to the type of structure to be designed. Design floods are determined from the frequency distribution of the corresponding flood data (Salas et al., 2013). Another important variable that can be obtained through frequency distribution studies is the risk. Risk is the probability that a certain project will fail during the lifetime of its use. According to Tingsanchali and Karim (2010), risk assessment, namely, identifying the possible impacts of any future flooding event, can help residents and authorities minimize economic and environmental losses.

The concepts of risk and uncertainty are commonly used for designing and evaluating flood-related hydraulic structures (Salas et al., 2013). A risk analysis should precede the sizing of projects that present a degree of complexity (such as unique works of art) before selecting the return period. In addition, hydraulic projects are generally designed by considering the minimum cost associated with an acceptable risk of failure. Therefore, the risk associated with damage caused by a hydrologic event higher than that predicted in the project, during the lifetime of its use, must be considered. This fact requires the prediction of hydrological quantities of significant magnitude, such as the maximum daily precipitation, which can cause flooding in a certain locality. According to Dave (2017), understanding the changes in extreme daily rainfall events is more important than realizing changes in the mean condition. The same author affirmed that 1-day extreme rainfall events occur not only in years of excess rainfall but also in years of drought. According to Jain and Kumar (2012), changes in rainfall from global warming require a review of the hydrologic design and management practices. Thus, it is necessary to develop studies that promote a better targeting of resources and technologies applied to disaster prevention.

Baptista et al. (2011) stated that, in Brazil, there are no specific regulations for the use of rainwater control devices in the cities; however, Brazilian legislation at federal, state, and municipal levels have legal instruments that can be used. Thus, the suggestions of manuals such as DNIT (2010), of the Departamento Nacional de Infraestrutura de Transportes (DNIT), and DAEE-CETESB (1986), of the Departamento de Águas e Energia Elétrica – Companhia de Tecnologia de Saneamento Ambiental (DAEE-CETESB), which present recommendations as to the return period to be adopted in the design of hydraulic projects, are based on a particular baseline.

This study focused on verifying if the probabilistic models of distribution, namely, Gumbel, LN, LP3 and Normal are efficient for an estimation of extreme events of precipitation for the recommended return period. This paper also discusses whether the methodology based on the return period that is used for the sizing of hydraulic projects in Brazil is adequate and safe for a tropical climate region. For this purpose, the cities of Amambaí, Iguatemi, Jardim, Naviraí, and Tacuru, located in the state of Mato Grosso do Sul, Brazil, which were affected by heavy rainfall and were declared as disaster area for a 180-day period in 2015, were adopted as a case study.

2. METHODOLOGY

2.1 Study area

The study area (Fig. 1) includes the cities of Amambaí, Iguatemi, Jardim, Naviraí, and Tacuru, located in the State of Mato Grosso do Sul, Brazil. Farming is the main economic activity of the region, which is characterized as an area of rural watersheds. The pedology of the study area is composed of orthic Quartzarenic Neosol (RQo), which is derived from sandstone rock, and of dystrophic Red Latosol (dRL) and eutrophic Red Argisol (PVe), both with a basalt origin.

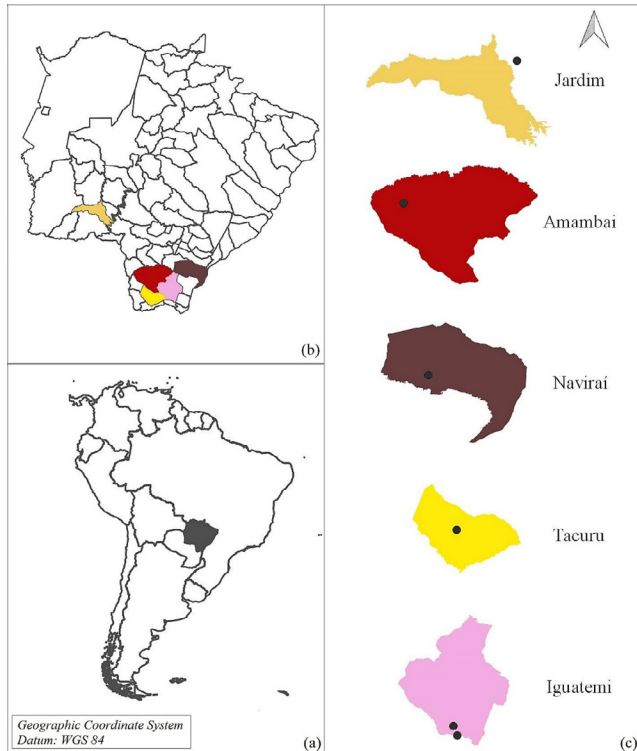


Figure 1 - Studied cities and locations of the measurement stations: a) State of Mato Grosso do Sul in South America, b) Studied cities in State of Mato Grosso do Sul, and c) Measurement stations in the studied cities

Source: Authors.

According to Köppen's climate classification, the prevailing climate in Jardim and Naviraí territory is Aw (Tropical savanna, wet), which is defined as hot and humid. This classification is attributed to regions where the climate is markedly seasonal, with rainfall of more than 250 mm per month between December and February (Alvares et al., 2013), showing well-defined rainfall periods with an average annual rainfall of around 1,500 mm. The months from November to April are the rainiest, whereas the driest period extends from July to September. The prevailing climate in Amambaí, Iguatemi, and Tacuru territory is Cfa (Humid subtropical climate), a humid temperate climate with hot summers. This classification is attributed to regions where summer and winter seasons

are well defined, with an annual average rainfall of between 1,500 and 1,700 mm, which is higher in the summer (> 600 mm) but may also occur during the winter (< 200 mm). The mean air temperature in the warmest month is higher than 22 °C (Alvares et al. 2013). The territorial area obtained by Instituto Brasileiro de Geografia e Estatística (IBGE), predominant soils, and climate of each city are presented in Table 1.

City	City territory (IBGE 2016)	Predominant Soil Class (Santos et al. 2011)	Predominant Climate (Alvares et al. 2013)
Amambaí	4.202 km ²	dystrophic Red Latosol (LVd27)	Cfa and Aw
Iguatemi	2.947 km ²	orthic Quartzarenic Neosol (RQo24)	Cfa
Jardim	2.201 km ²	eutrophic Red Argisol (PVe32)	Aw
Naviraí	3.193 km ²	dystrophic Red Latosol (LVd43)	Aw
Tacuru	1.786 km ²	dystrophic Red Latosol (LVd26 e LVd27)	Cfa

Table 1 - Study area characteristics

Source: Authors.

2.2 Hydrological data

Dense rain-gauge networks, high temporal-resolution sampling, and long-term data collection are all necessary to capture extreme and infrequent events (Keefer et al., 2016). The hydrological data series used in this study was collected from the Agência Nacional de Águas (ANA) database. In each city, the station with the longest measurement period was selected to be used in the estimation of maximum rainfall (Table 2). Datasets of telemetry station 64725000, whose operator is Itaipu, were used to obtain the rainfall height in the period that was recorded disaster by heavy rains in the city. The hydrological data were prepared and worked on using an electronic spreadsheet.

Station code	City	Latitude	Longitude	Size of data series
2355000	Amambaí	23,10° S	55,24° W	47 years
2354001	Iguatemi	23,68° S	54,56° W	47 years
2156001	Jardim	21,44° S	56,09° W	51 years
2354000	Naviraí	23,06° S	54,19° W	47 years
2355003	Tacuru	23,64° S	55,02° W	35 years

Table 2 - Stations characteristics.

Source: Authors.

2.3 Disaster data

A disaster is characterized as the result of adverse, natural, or human-made adverse events in a vulnerable environment, causing serious disruption to the functioning of a community or society involving extensive human, material, economic, or environmental loss and damage exceeding the region's ability to cope with the problem through its means (BRAZIL, 2012). The Brazilian System of Civil Protection and Defense (SINPDEC) comprises a group of municipalities affected by natural or technological events that trigger damage and losses that characterize a disaster. This information is obtained through the Disaster Information Form (FIDE), which is regulated by Normative Instruction No. 2 from 12/20/2016 (BRAZIL, 2016).

In this study, the starting point of the investigation was the generation of this form, allowing the damage and losses to be measured. The State Coordination of Civil Defense of Mato Grosso do Sul (CEDEC/MS) database has information inserted in the FIDE, as adopted by SINPDEC, by municipalities affected by natural or technological events that triggered damage and losses characterizing a disaster. The dataset contained in the FIDE was used to characterize the susceptibility of the region to the events of maximum precipitation. The losses and damage data recorded in the FIDEs were used to verify which projects were susceptible to collapse when faced with extreme tropical rainfall events and investigate their dimensioning normative criteria.

2.4 Structural failure risk and return period recommended for hydraulic projects

A risk assessment requires a description of the probabilistic properties of the hydrological variables (Renard and Lang, 2007). According to Şen (1999), the risk (R) can be defined as the probability of occurrence of a hydrological variable being greater than the design magnitude at least once over the system's economic life (equation 1).

$$R = 1 - (1 - 1/T)^n \quad (1)$$

Where T is the return period or recurrence interval in years, n is the number of years of use of the facility or the lifetime of its use, and R is the risk given in percentage.

The calculated risks are very important in the design of a structure (Tung and Wong 2014), and are commonly used to design and evaluate flood-related hydraulic structures (Salas et al., 2013; Tingsanchali and Karim, 2010). According to the calculated risk for the probable lifetime of a structure, it possible to observed that, for small return periods, the designer assumes a high risk of

structural collapse during a natural disaster, such as an extreme rainfall event. Researchers (Tingsanchali and Karim, 2010; Gebregiorgis and Hossain, 2012) have conducted studies in different countries aiming to minimize structural failures. Tingsanchali and Karim (2010) contributed to a comprehensive assessment of flood hazards and risk for the Phrae flood plain of the Yom River basin in northern Thailand. Gebregiorgis and Hossain (2012) conducted a reassessment of the oldest dam in the Tennessee River basin, the Wilson Dam, based on post dam flow data.

Recommendations have been made regarding the use of the return period for hydraulic design and other structures. In Brazil, a recommended standard includes the Manual of DNIT (2010), which contains many recommendations about the return period of hydraulic structures (Table 3).

Hydraulic Structures	Return Period (years)
Surface Drainage	5 to 10
Subsurface Drainage	10
Tubular culverts	15 (as a channel)
	25 (as an orifice)
Cellular culverts	25 (as a channel)
	50 (as an orifice)
Small bridge (Pontilhão)	50
Bridge	100

Table 3 - Return period of hydraulic structure for DNIT (2010).
 Source: DNIT (2010).

There are other recommendations used for dimensioning in Brazil, including DAEE-CETESB (1986) and Naghettini and Horizonte (1999) (Table 4).

Hydraulic Structure	Characterization	Return period (years)
Road culverts	Low traffic	5 to 10
	Intermediate traffic	10 to 25
	High traffic	50 to 100
Road bridges	Secondary road	10 to 50
	Primary road	50 to 100
Urban drainage	Small cities drainage gallery	2 to 25
	Major cities drainage gallery	25 to 50
	Channeling stream	50 to 100
Dike	Rural area	2 to 50
	Urban area	50 to 200
Dams	No risk of loss of human life	200 to 1000
	At risk of loss of human life	10000

Table 4 - Return period for hydraulic structures.
 Source: Naghettini and Horizonte (1999).

The Porto (1995) recommendations consider the characteristics of the area to estimate the return period in a project (Table 5).

Work	Occupation area	T (years)
Micro drainage	Residential	2
	Commercial	5
	Areas with public service buildings	5
	Airports	2 – 5
	Commercial areas and traffic arteries	5 – 10
Macro drainage	Commercial and residential areas	50 – 100
	Areas of specific importance	500

Table 5 - Return period for different types of occupation areas.
Source: Porto (1995).

2.5 Probabilistic analysis

An estimation of the rainfall for a desired return period, and for different durations, is often required for the design of hydraulic and other types of structures in a region, which can be achieved through a probabilistic approach (Vivekanandan and Mathew, 2010). Thus, theoretical probability distribution models are often applied to obtain an extrapolation of extreme events for high return periods. Yue and Hashino (2007) affirmed that, for an effective plan, design, and management of water resource engineering, such as an urban water supply, hydropower, or irrigation system, data on the precipitation over longer durations, such as annually, seasonally, or monthly, are necessary. To prevent property and human life loss, designs of hydraulic and hydrologic structures are based on extreme rainfall estimates (Kao and Govindaraju, 2007). According to Renard and Lang (2007), the severity of an extreme hydrological event is expressed as the non-exceedance probability, or equivalently, in terms of the return period. Dawdy and Matalas (1964) define the return period as the average interval of time within which the magnitude of the event will be equaled or exceeded once. The inverse of the return period is the exceedance probability. The precipitation levels of the annual series are listed in decreasing order, associating their respective probability of occurrence (equation 2) and the return period (equation 3).

$$p = m / (n + 1) \tag{2}$$

$$T = 1 / p \tag{3}$$

Where p is the cumulative probability of an event being matched or exceeded in magnitude, m is the order number, n is the total number of years considered, and T is the return period or recurrence interval in years.

According to Salvadori and De Michele (2007), the return period of hydrological events represents a common criterion for sizing a hydraulic engineering project, and provides a simple, yet efficient, means for conducting a risk analysis. In this study, the annual maximum daily rainfall was arranged in decreasing order of magnitude and plotted on a logarithmic scale with their respective return periods. To analyze the maximum precipitation, Gumbel, Log-Normal (LN), Log-Pearson Type III (LP3), and Normal distribution models have been applied for the return periods.

Applying these distribution models to a set of observations of a hydrological variable requires a simple random sample drawn from a single population (Naghetini and Pinto, 2007). Thus, in order to be able to analyze the frequency of rainfall, initially the independence, homogeneity and stationarity of the data series were verified. The verification was performed using the following non-parametric tests: Wald and Wolfowitz (1943) test to verify independence; Mann and Whitney test (1947) to analyze homogeneity; and Spearman test, described by NERC (1975), to verify stationarity.

The Gumbel distribution (equations. 4 to 6), is used by the United States National Weather Service (NWS) to adjust the extreme values of rainfall height to be used on maps, which relate the rainfall height variation to an affected area (Silva et al. 2003).

$$\beta = 6^{0.5} \cdot S / \pi \tag{4}$$

$$\alpha = (\mu - 0,5772 \cdot \beta) \tag{5}$$

$$x(T) = \beta - \alpha \cdot \ln(-\ln(1-1/T)) \tag{6}$$

Where S is the standard deviation, μ is the mean, and α and β are the parameters of the Gumbel distribution obtained using the moments method (Naghetini and Pinto 2007).

The function of the Normal distribution (equation 7) can be expressed in a simplified form in equation (8) with a reduced variable (z), which is associated with a non-exceedance probability of a standard normal distribution (equation 9).

$$f_x(x) = 1 / \sqrt{2\pi\sigma_x} \exp[-1/2 ((x - \mu_x) / \sigma_x)^2] \tag{7}$$

$$x = \mu_x + z \cdot \sigma_x \tag{8}$$

$$z = [(x - \mu) / \sigma] \tag{9}$$

Where x is the maximum precipitation for a given probability, and σ is the standard deviation of the annual maximum daily precipitation observed.

Among the probability distributions, LN in the equation (10) has been adjusted well for the Brazilian rainfall distribution (Longo et al., 2006).

$$\log(x) = \overline{\log(x)} + z \cdot \sigma_{\log x} \quad (10)$$

Where $\log(x)$, $\overline{\log(x)}$, and $\sigma_{\log x}$ are the logarithm, and the average and standard deviation of the logarithms of the annual maximum daily precipitation observed, respectively.

The LP3 distribution has been one of the most frequently used distributions for hydrologic frequency analyses since the recommendations of the Water Resources Council (1967, 1982) of the United States regarding its use as the base method (Singh 1998). The LP3 distribution (equation 11) is calculated in data analysis to estimate the maximum precipitation with a determination of the frequency factor K_p (Table 6), considering the asymmetry coefficient g , equation (12).

$$\log(x) = \overline{\log(x)} + K_p \cdot \sigma_{\log x} \quad (11)$$

$$g = [n \sum (x_i - \bar{x})^3] / [(n-1) \cdot (n-2) \cdot \sigma_{\log x}^3] \quad (12)$$

To analyze the adherence of theoretical probability distributions to the data series, it was used the Kolmogorov-Smirnov test (KS) (Massey Jr., 1951), adopting a significance level of 5%.

Asymmetry coefficient	Return period (years)							
	2	5	10	25	50	100	200	1000
-0,1	0,017	0,836	1,270	1,716	2,000	2,252	2,482	2,950
-0,2	0,033	0,850	1,258	1,680	1,945	2,178	2,388	2,810
-0,3	0,050	0,853	1,245	1,643	1,890	2,104	2,294	2,675

Table 6 - Factor K_p related to the return period and the asymmetry coefficient.
 Source: Authors.

3. RESULTS AND DISCUSSION

Through the annual maximum precipitation series it was verified, at a significance level of 5%, the independence, homogeneity and stationary conditions of the data series of each station (Table 7).

After verifying the independence, homogeneity and stationarity conditions of the data series, it was analyzed the adequacy of theoretical probability distributions to empirical distributions (Fig. 2).

Station code	Distribution	T (years)							
		2	5	10	25	50	100	200	1000
2355000	Normal	85,69	106,11	116,79	128,18	135,53	142,15	148,20	160,68
	Log-Normal	82,12	106,05	121,22	139,79	153,27	166,51	179,62	210,01
	Gumbel	81,70	103,15	117,35	135,29	148,60	161,81	174,98	205,47
	Log-Pearson III	84,62	106,50	118,24	130,71	138,66	145,67	151,93	164,30

In this study, due to the fact that the neperian logarithm of the data series presented negative asymmetry coefficient, it was not possible to estimate the KS for the adherence between the empirical distribution and LP3. The Normal, Log-Normal and Gumbel distributions fit all the data series analyzed (Table 8).

After checking the adherence, the expected maximum precipitation for the recommended return periods for the different hydraulic structures for each city was estimated (Table 9). For relatively short return periods, the LP3 and the Normal distribution presented the highest level of estimated precipitation. However, for large periods of return, the maximum precipitation was estimated by the Gumbel and LN distributions.

Station code	City	Independence test	Homogeneity test	Stationarity test
2355000	Amambaí	Independent	Homogeneous	Shows no temporal tendency
2354001	Iguatemi	Not independent	Homogeneous	Shows temporal tendency
2156001	Jardim	Independent	Homogeneous	Shows no temporal tendency
2354000	Naviraí	Independent	Homogeneous	Shows no temporal tendency
2355003	Tacuru	Independent	Homogeneous	Shows temporal tendency

Table 7 - Independence, homogeneity and stationarity tests results of each station.
 Source: Authors.

Station code	City	Critical value	Normal distribution	Log-Normal distribution	Gumbel distribution
2355000	Amambaí	0,198	0,063	0,107	0,103
2354001	Iguatemi	0,201	0,117	0,098	0,082
2156001	Jardim	0,207	0,097	0,080	0,068
2354000	Naviraí	0,198	0,097	0,072	0,081
2355003	Tacuru	0,224	0,120	0,071	0,084

Table 8 - Kolmogorov-Smirnov test result of each station.
 Source: Authors.

2354001	Normal	93,28	116,63	128,84	141,86	150,27	157,83	164,76	179,03
	Log-Normal	89,44	114,54	130,34	149,61	163,54	177,18	190,65	221,76
	Gumbel	88,72	113,24	129,48	150,00	165,21	180,32	195,37	230,24
	Log-Pearson III	89,77	114,65	130,02	148,46	161,60	174,31	186,73	214,91
2156001	Normal	90,17	116,60	130,42	145,15	154,67	163,23	171,06	187,22
	Log-Normal	84,67	115,86	136,49	162,56	182,00	201,45	221,07	267,77
	Gumbel	85,02	112,77	131,14	154,36	171,58	188,68	205,71	245,17
	Log-Pearson III	88,58	116,42	131,22	146,69	156,33	164,68	171,98	185,87
2354000	Normal	106,56	132,13	145,49	159,75	168,95	177,24	184,82	200,44
	Log-Normal	102,20	131,66	150,29	173,08	189,61	205,82	221,87	259,01
	Gumbel	101,57	128,42	146,19	168,65	185,32	201,86	218,33	256,51
	Log-Pearson III	105,86	132,19	145,79	159,81	168,47	175,92	182,43	194,77
2355003	Normal	96,32	117,08	127,93	139,49	146,97	153,69	159,84	172,53
	Log-Normal	93,24	116,21	130,38	147,41	159,57	171,37	182,93	209,28
	Gumbel	92,27	114,07	128,49	146,72	160,25	173,67	187,05	218,03
	Log-Pearson III	94,27	116,50	129,36	143,99	153,95	163,24	172,01	190,86

Table 9 - Expected precipitation for different return periods calculated through theoretical probability distributions for each station.

Source: Authors.

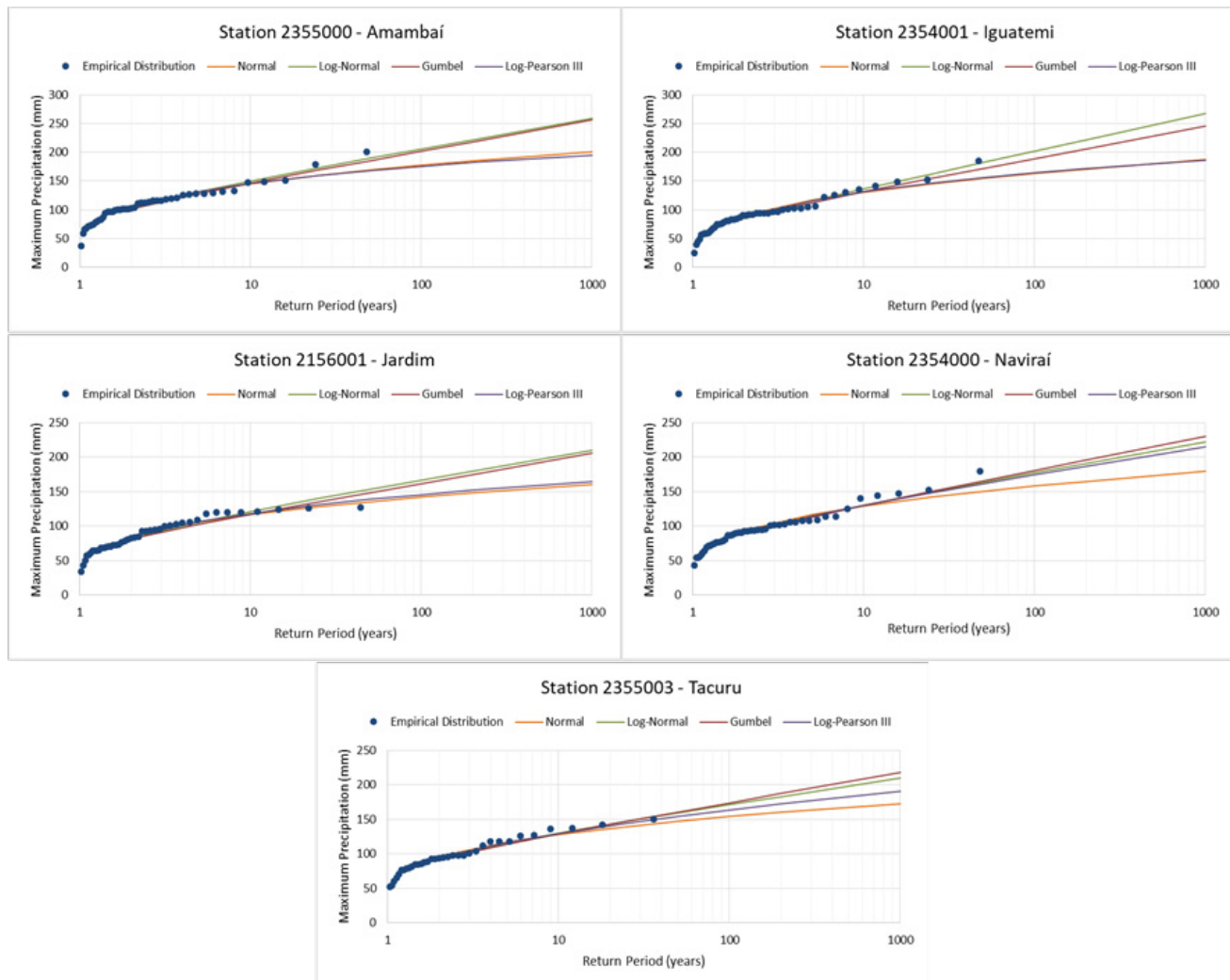


Figure 2 - Adjust Normal, Log-Normal, Gumbel and Log-Pearson III distributions to data series.

Source: Authors.

Disasters related to heavy rainfall occurred and were recorded in official documents during the last half of November of 2015. The Official Diary of the State of Mato Grosso do Sul, No. 9.059 declared an emergency situation in the study region. The cities of Amambai, Iguatemi, Naviraí and Tacuru experienced a major disaster during a period of 180 days in 2015 (Mato Grosso do Sul, Decree Law "E", Nº 35, on December 3 2015), which according to information provided in a FIDE (1.3.2.1.4, IN/MI 01/2012), was classified and codified as heavy rainfall. The FIDE reported that urban, rural, commercial, agricultural, and livestock areas were affected by extreme rainfall events. Strong winds and floods caused the destruction of streets, avenues, bridges, flooding of homes, and crop losses. The storm caused damage to some homes, left families homeless, and destroyed roofs, injuring the residents. The cities most affected by heavy precipitation were Amambai, Iguatemi, and Jardim. In Naviraí, the FIDE recorded erosion, ruptures, and damage to the sewage and water networks. The heavy rains caused electric power poles to fall, flooding in some homes, and runoff from streams. In Tacuru, a similar situation occurred, and erosion and landslides were recorded in large portions of the area. The highways were compromised and bridges were damaged, altering the daily lives of the people, preventing the flow of production, compromising local commerce, and making it impossible to provide transportation services and health care, inconveniencing the population and causing significant damage to the city.

At Estrada/Iguatemi Station, code 64725000, operated by Itaipu, 150.6 mm of rainfall was registered during a 24-h period between 23 and 24 November 2015. This station is located approximately 6.4 km from the station operated by ANA, where data were obtained from calculations of the maximum return periods. According to the return periods recommended in the standards, the probabilistic distribution models were able to predict the extreme rainfall events of 98.60 mm for Amambai and 81.20 mm for Jardim.

All the distributions models were able to predict the extreme event of 140 mm occurred in Naviraí for 25 years of the return period. Gumbel and LN distribution models were able to predict the extreme rainfall events of 150.6 mm in Iguatemi for 25 years of return periods, and Normal and LP3 for 50 years of return period. Thus, the probabilistic distribution models tested were not able to predict the extreme rainfall events that occurred in the Tacuru region (185.3 mm) during the recommended return periods of the standards. Furthermore, the probability models were able to predict the occurrence of intense rains in the cities of

Tacuru only when considering a high return period. For a return period of over 200 years, the Gumbel distribution estimated the extreme precipitation that occurred in Tacuru. The precipitation was estimated for a T equal to 1,000 years when using LP3 and LN. Among the analyzed models, the Normal distribution proved to be the least efficient.

According to Koutsoyiannis (2004), the Gumbel distribution has been the prevailing model for rainfall extremes despite the fact that it results in the highest possible risk for engineering structures. Besides, the author shown that hydrological records of typical length (some decades) may display a distorted picture of the actual distribution, suggesting that the Gumbel distribution is an appropriate model for rainfall extremes while it is not.

The results obtained in this work illustrate that even the models being able to predict the precipitations occurred for short return periods, were not sufficient to avoid the occurrence of damage to the structures. Probabilistic distribution models were able to predict the extreme rainfall of 98.60 mm for Amambai and 81.20 mm for Jardim, for the recommended return periods. In the cities of Amambai and Jardim, the return period suggested by the manuals for sizing damaged structures predicted the maximum rainfall, but was not sufficient to prevent damage to structures as recorded in the FIDEs of these cities. The return periods suggested by DNIT (2010) failed to predict rainfall that caused damage to manholes and urban drainage systems in the cities of Iguatemi and Naviraí, yet predicted maximum precipitation for bridges, but was unsuccessful in preventing damage to structures. For the city of Tacuru, the design recommendations were unable to predict precipitation that caused damage to bridges, culverts and urban drainage. These intense precipitations could not be estimated, requiring alternative measures that offer greater protection to the population.

This fact indicates the importance of inserting new parameters for the structural dimensions because increasing only the time of return makes the projects too expensive. Thus, alternative measures that offer adequate protection to the population are necessary. In the case of China, which has more than 5,000 years of experience in flood management policy, a form was implemented to prevent such future events using structural solutions allied with non-structural techniques, including changes in land use, welfare, removal in vulnerable areas, and environmental protection (Luo et al., 2015). Countries like South Africa have experienced similar situations, in which most of the methods used, developed between 1960 to

1970 (HRU, 1972), are already out-dated and need to be updated to meet the current conditions for estimating projected flooding (Görgens, 2007; Smithers et al., 2015).

The lifetime use of a manhole according to Tung et al. (2006) is 35 years. In this study it was verified that even damaged projects were dimensioned according to the recommendations of the NBR 15575 (2013), considering a lifetime use of 50 years, which is common for structural works, and the risk of failure was assumed to be high. Furthermore, the lower the return period applied, the higher the risk that is assumed. In addition, the return periods suggested through the Brazilian regulations have not considered the frequent occurrence of intense precipitation in the face of global climate change.

According to DNIT (2010), the risk assessment already includes the occurrence of extreme situations since the return period of large structures is great. However, external situations can generate structural faults interfering in the return period of the structure. In the bridges, assuming a correct dimensioning, the main interference is the flood, due to the erosive action around the pillars, which causes the wear of the structure. According to Selvakumaran et al. (2018) the most common cause of bridge collapse is due to the removal of ground material at its piers during flooding, some failures causing loss of life and most resulting in significant transport disruption and economic loss.

In other countries that use the return period as the index in their classes, the return period is based primarily on historical data or previous practical applications, or on a comparison of statistical tests. According to Ren et al. (2017), in China and Russia, a Pearson Type III distribution is used, whereas, in Canada and India, a Logarithmic and Pearson Type III distribution is applied.

DAEE-CETESB (1986) and Naghettini and Horizonte (1999) recommend that the return period varies from 5 years for a culvert construction to 10,000 years for major dam construction. Gebregiorgis and Hossain (2012) affirmed that there are different guidelines for selecting the return period or flooding of hydraulic structures in the design used by different agencies.

According to Ren et al. (2017), in the United States, a frequency analysis was adopted for reservoir projects until 1930, and from 1938 the hydrometeorology method began being used. The United States Army Corps of Engineers (USACE) proposed a risk classification during the 1970s. The most common methods applied are flood frequency analysis and hydrometeorology. In South Africa, a drainage manual (SANRAL 2007) is used. In this manual, the methods used to estimate the project

floods are based on empirical, deterministic, and probabilistic approaches. When the records are inadequate, or there is simply no flow data available at the site of interest, the unit hydrograph, Rational Method, and SCS methods, which are based on rainfall-flow events for flood estimation in South Africa, are recommended (Smithers, 2012). However, Van der Spuy and Rademeyer (2010) affirmed that the search continues in South Africa for a universally applicable design flood estimation method. Such searches include a rational method based on probability, called a Flood Standard Design (SDF), developed by Alexander (2001); the Joint Peak-Volume (JPV) developed by Görgens (2007); and the Regional Estimation of Extreme Flood Peaks by Selective Statistical Analysis (REFSSA), developed by Nortje (2010).

4. CONCLUSIONS

Tropical climate regions are characterized for their heavy rainfall events during the summer, and in recent years, as a result of climate change, such events are becoming recurrent.

The cities selected for the study area were affected by heavy rainfall in November 2015 and experienced a disaster that lasted for a period of 180 days. The rainfall event caused significant damage and losses, and different types of structures were affected, mainly hydraulic.

In this paper, we discussed whether the probabilistic distribution models are adequate for predicting extreme rainfall events for regions with climate characteristics to the study area. Therefore, we concluded that the Gumbel, LN, LP3, and normal probabilistic distribution models are not recommended to design the infrastructure found in the studied cities.

The probability models were able to predict the occurrence of extreme rainfall in the cities of Iguatemi and Tacuru only when considering a high return period.

Statistical analysis showed that the data series of the city of Iguatemi and Tacuru showed a temporal trend. The fact that the series are not stationary demonstrates the scarcity of extensive data sets available.

ACKNOWLEDGMENTS

The authors are grateful to the Coordenação de Aperfeiçoamento de Pessoal de Nível Superior – CAPES, to the Conselho Nacional de Desenvolvimento Científico e Tecnológico – CNPq, to the Programa de Apoio à Pós-graduação - PROAP and to the Federal University of Mato Grosso do Sul - UFMS for their support in the development of this work.

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COMO CITAR ESTE ARTIGO

OLIVEIRA, Ana Clara Costa; ALMEIDA, Aleska Kaufmann; GUARIENTI, José Antonio; LIMA, Cleyse Andreia Souza; ALMEIDA, Línica Vicente Ferreira de; SOUZA, Robert Schiaveto de; ALMEIDA, Isabel Kaufmann de. Extreme Precipitation Events And Associated Risk Of Failure In Hydraulic Projects In The State Of Mato Grosso Do Sul, Brazil. **MIX Sustentável, [S.l.], v. 7, n. 2, p. 147-160,**

abr. 2021. ISSN 24473073. Disponível em:<<http://www.nexos.ufsc.br/index.php/mixsustentavel>>. Acesso em: dia mês. ano. doi:<https://doi.org/10.29183/2447-3073.MIX2021.v7.n2.147-160>.

DATA DE ENVIO: 13/10/2020

DATA DE ACEITE: 11/03/2021

